

SEMI- VS. FULLY-DISTRIBUTED URBAN STORMWATER MODELS

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Abstract

Urban stormwater models are important tools for flood simulation, prediction and management. Their main input is rainfall data, the temporal and spatial resolution of which must be in agreement with that of the hydrological/hydraulic model. Fully distributed (FD) urban stormwater models are generally more detailed and physically-based than the commonly used semi-distributed (SD) models. In this work a comparison is presented of SD and FD models based on two real case studies in UK and Portugal.



SD and **FD** models



SD models are based on sub-catchment discretisation, through which rainfall (assumed to be uniform within each subcatchment) is applied to the model. Runoff volumes are estimated and routed within each subcatchment based on empirical or conceptual methods.

FD models are based on two-dimensional representations of the overland surface, where the rainfall is applied directly onto each mesh element and runoff volumes are applied and routed.

A methodology to build comparable SD and FD models was developed and similar data were collected at both case studies.



Cranbrook case study, London, UK

The comparison between the SD and FD model of Cranbrook case study was based on 3 storm events for which local rainfall and runoff data were available:

| | | Rainfall | | | | |
|--------|------------------|------------------|--------------|--------------------------------|------------|--------------------------------|
| Event | Start | End | Duration (h) | Maximum intensity (mm/h) | Total (mm) | Average intensity (mm/h) |
| 141212 | 12/12/2014 01:30 | 12/12/2014 08:00 | 6.5 | 12 | 10.9 | 2 |
| 150103 | 03/01/2015 03:50 | 03/01/2015 17:00 | 13.2 | 12 | 16.6 | 1 |
| 150108 | 08/01/2015 07:30 | 08/01/2015 14:30 | 7.0 | 12 | 11.6 | 2 |

Volume balance (generated runoff, overland storage & outfall discharge):

| | | | Runoff volume (m3) | Diference of the v | water volume on the 2D surface (final-i | nitial; m3) ∎Vol | ume discharged |
|--|------------------|---------|--------------------|--------------------|---|------------------|----------------|
| Runott volumes | (7h) | 1D2D SD | | 24370 | 1386 | 230 | 08 |
| generated in both models are very similar. | 2015/1/8 | 1D2D FD | | 27275 | | 14985 | 1 |
| | /1/3 2h) | 1D2D SD | | 33920 | 49 | 3210 | 0 |
| The FD model has a | 2015, (13.2 | 1D2D FD | | 37831 | 1 | 9738 | 17 |
| significantly higher | 2/12 | 1D2D SD | | 21275 | 1353 | 192 | 271 |
| overland storage, which translates into smaller | 2014/12 (6.5h | 1D2D FD | | 23732 | | 14399 | |
| discharge through outfalls | . 50 | 1D2D SD | | 28165 | 2764 | 2 | 4042 |

Zona Central case study, Coimbra, Portugal

1D2D FE

1D2D SD

1D2D FD

1D2D SD

1D2D FD

The comparison between the SD and FD model of Zona Central case study was based on 4 flooding events:

| | | Poturn | | Rainfall | | | | |
|-------|----------------|--------|------------------|------------------|--------------------------------|------------|--------------------------------|------|
| Event | period (yr) | Start | End | Duration (h) | Maximum intensity (mm/h) | Total (mm) | Average intensity (mm/h) | |
| | 060609 | 50 | 09/06/2006 14:50 | 09/06/2006 16:30 | 1.7 | 144.0 | 36.6 | 22.0 |
| | 061025 | 50 | 25/10/2006 00:30 | 25/10/2006 05:30 | 5.0 | 102.0 | 56.6 | 11.3 |
| | 080921 | 5 | 21/09/2008 15:10 | 21/09/2008 17:20 | 2.2 | 60.6 | 21.4 | 9.9 |
| | 131224 | 5 | 24/12/2013 06:40 | 24/12/2013 18:00 | 11.3 | 31.5 | 48.9 | 4.3 |

Volume balance (generated runoff, overland storage & outfall discharge):

| Same as in Cranbrook, | 2/24 |
|------------------------|----------------|
| the FD model stores | 2013/1 |
| more runoff in the | 21 |
| overland. However, in | 2008/9/ |
| this case study, the | |
| difference between |)(10/2 (도노) |
| both models is smaller | 20 |

| Kulloff volume (m5) Difference (| of the water volume on the 21 | (1, 1115) = volu | • volume discharged by outlans (m3) | | |
|----------------------------------|-------------------------------|------------------|-------------------------------------|-------|--|
| 53939 | 14195 | | 40899 | | |
| 48039 | 48039 | | | 27881 | |
| 22991 | | 8473 | 14619 | | |
| 20864 | | 11377 | | 9182 | |
| 62286 | | 17587 | | 45134 | |
| 57492 | | 23964 | | 33010 | |
| | | | | | |

uischarge through outlails 👙 🕫 ⁷07 ' 1D2D FD of the sewer system.

Difference in maximum overland volumes for each main land use group type:

FD model has higher overland volumes on roads and on urbanised areas. This is due to water being retained in singularities within the 2D overland model (e.g. around buildings)



13737

13759

Simulated vs. Observed water depth and flow in sewers:



than in Cranbrook. 18615

Difference in maximum overland volumes for each main land use group type:

The FD model shows higher flooding volumes in residential areas as compared to the SD model. This is likely because private (building) connections to the sewer system are not represented. Therefore, runoff generated on the 2D overland model never reaches the sewer system and stays on the overland.



This suggest that correct implementation of FD models requires higher detail of the sewer network.

Floodplains at the city centre (8 de Maio Square) for the events with highest

return period:

Floodplains on the 8 Maio Square are well captured by the SD model and underestimated by the FD one. Due to sparse details of sewer inlets and private connections, in the FD model surface runoff volumes are retained in upstream areas and





Conclusions

Results suggest that FD models are more sensitive to surface storage and their implementation requires higher detail of the sewer network. Failure to represent the sewer system in detail may lead to misrepresentation of runoff processes in FD models. When high-resolution data are not available, the use of SD models could be a better choice, or a combination of SD on urbanized areas with FD models on open areas could be applied.

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